

units

Norton Tables 1-4 and 1-5

hardness

Norton Table 2-3

material properties

Norton Appendix A-1 general E G ν ρ

Norton Appendices A-2 and A-3 aluminum

Norton Appendix A-9 steel



6-32 UNC bolts $A_T = 0.0091 \text{ in}^2$ Norton Table 15-1

guess 1010 cold rolled steel $S_{UT} = 53 \text{ ksi}$ Norton Table A-9

$$F_{\text{BOLT}} r = F_{\text{GRIP}} R \quad F_{\text{BOLT}} = F_{\text{GRIP}} R / r \quad \tau \approx \frac{F_{\text{BOLT}}}{A_T} = \frac{F_{\text{GRIP}} R}{r A_T}$$

$$\text{von Mises } \sigma' = \sqrt{3} \tau = \frac{\sqrt{3} F_{\text{GRIP}} R}{r A_T}$$

at failure assume $S_{UT} = \sigma'$

$$F_{\text{GRIP}} = \frac{r A_T S_{UT}}{\sqrt{3} R} = \frac{(0.55 \text{ in})(0.0091 \text{ in}^2)}{\sqrt{3} (6 \text{ in})} \left(\frac{53000 \text{ lbf}}{\text{in}^2} \right) = 25.53 \text{ lbf}$$

σ normal stress
direct tension/compression
 $\sigma > 0$ tensile
 $\sigma < 0$ compressive
bending
contact
direct load bearing
Hertzian contact

τ shear stress
direct shear
torsional shear
pull-out shear
beam shear

Mohr's circle

σ' von Mises equivalent stress

S strength
yield
ultimate
fatigue

$N = S / \sigma'$ factor of safety

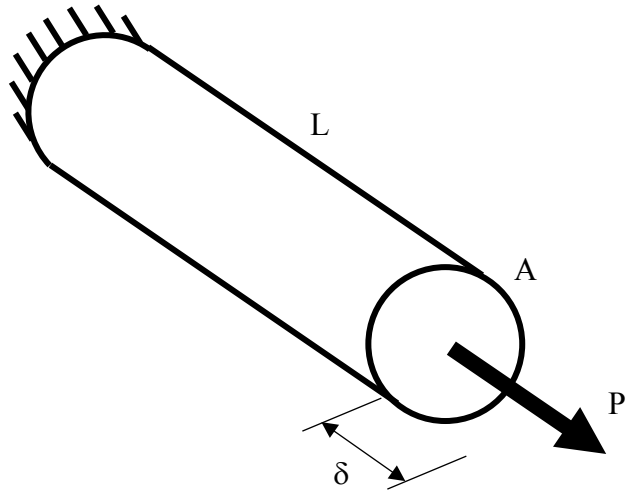
ϵ strain
units [μ inch per inch]

normal stress

axial tension or compression (Section 4.7 Norton)

uniform across cross-section, uniform along length, independent of cross section shape

$$\sigma = P / A \quad \delta = \frac{P L}{A E}$$



normal stress

simple cantilever bending (Sections 4.9 and 4.10 Norton)

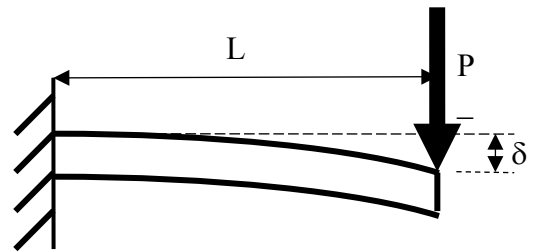
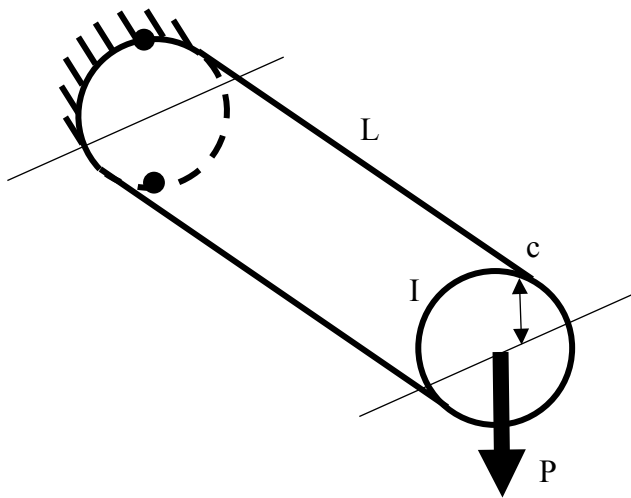
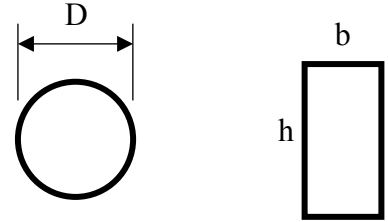
zero at neutral axis, maximum at top and bottom, linear along length

$$\sigma = \frac{M c}{I} \quad \delta = \frac{P L^3}{3 E I}$$

c_{MAX} at top and bottom $M_{MAX} = P L$ at fixed end

circular $c_{MAX} = D/2$ $I = \pi D^4 / 64$

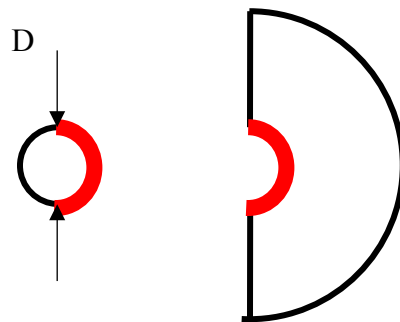
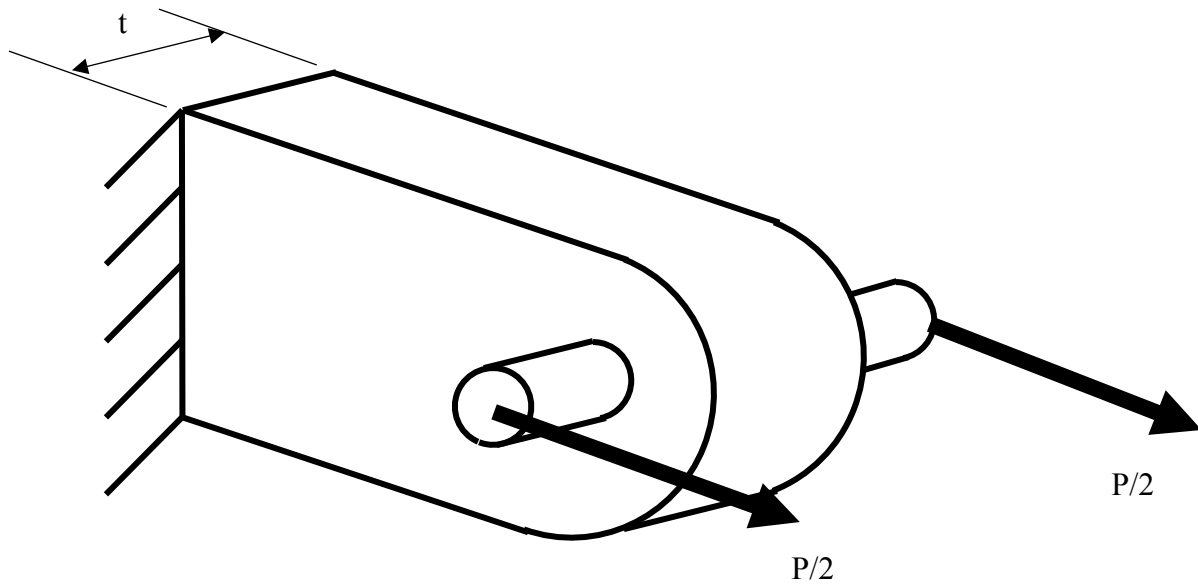
rectangular $c_{MAX} = h/2$ $I = b h^3 / 12$



normal stress

direct contact compression (Section 4.8 Norton)

$\sigma = P / A$ projected area tight fit $A = D t$ loose fit $A = \pi D t / 4$



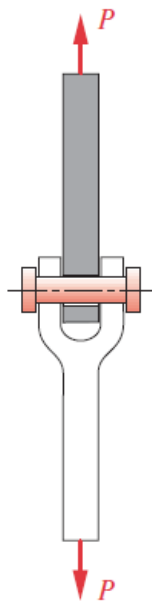
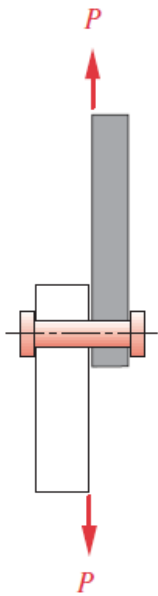
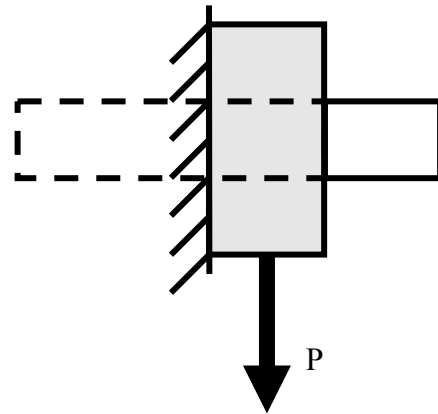
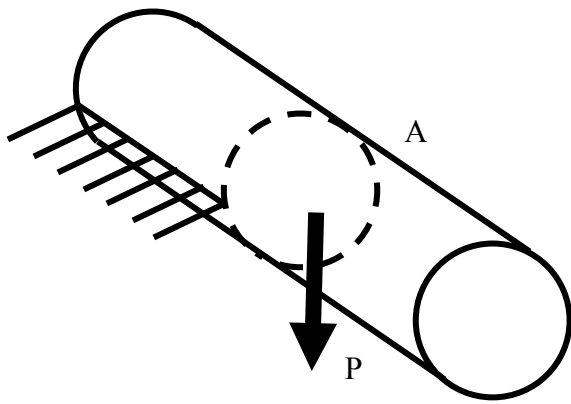
shear stress

direct shear (Section 4.8 Norton)

uniform across cross-section

$$\tau = P / A$$

(dual direct shear for clevis pins)



(a) Pivot in single shear
(less than ideal)

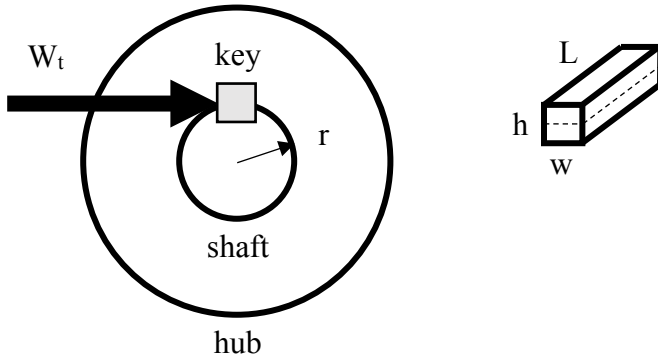
(b) Pivot in double shear
(preferred)

shear stress

direct shear for key

uniform across cross-section

$$T = W_t r \quad \tau = W_t / A \quad A = w L$$



shear stress

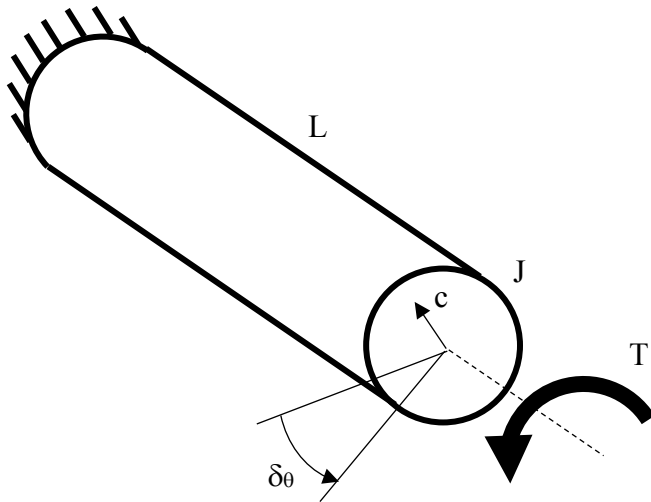
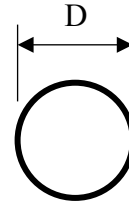
torsional shear (Section 4.12 Norton)

zero at center, maximum outside, uniform along length

$$\tau = \frac{T c}{J} = \frac{16 T}{\pi D^3} \quad \delta_{\theta} = \frac{T L}{J G} \quad \text{only for circular cross-section}$$

c_{MAX} at outside T constant along length

circular $c_{MAX} = D/2$ $J = \pi D^4 / 32$

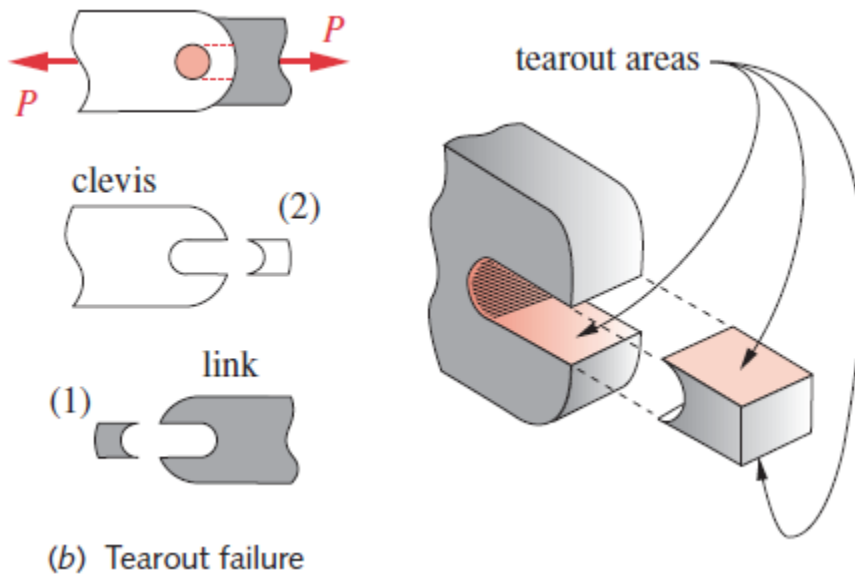


shear stress

pullout shear (Section 4.8 Norton)

uniform across cross-sections

$$\tau = P / A \quad A = 2 h t \quad h = \sqrt{R_o^2 - R_i^2}$$



shear stress

beam shear (Section 4.9 Norton)

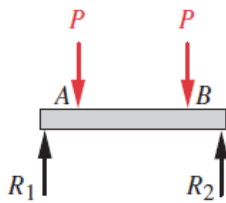
maximum at neutral axis, zero at top and bottom, varies along length

$$\tau = \frac{VQ}{Ib} \quad Q = \int_{y_1}^c y \, dA$$

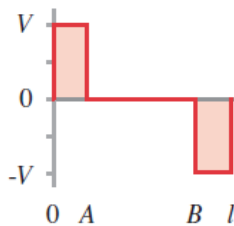
circular $\tau_{\text{MAX}} = \frac{4V}{3A}$

rectangular $\tau_{\text{MAX}} = \frac{3V}{2A}$

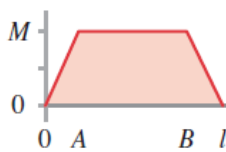
bending is maximum at top and bottom, zero at neutral axis



loading diagram



shear diagram



moment diagram

combined stress

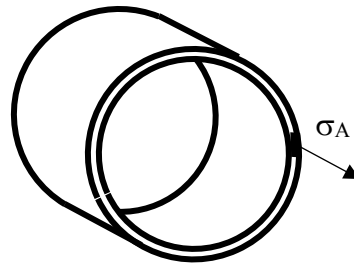
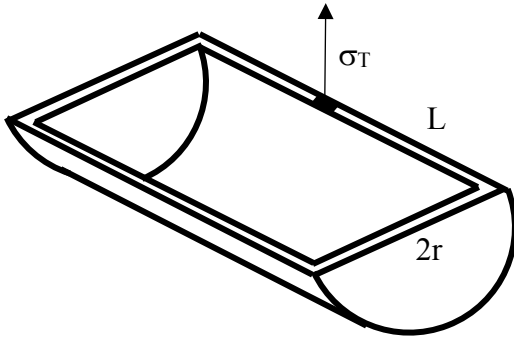
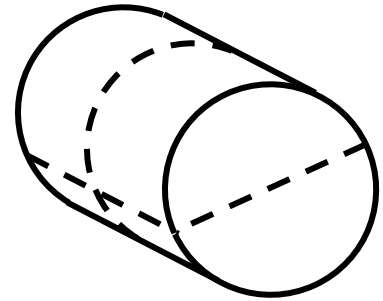
thin wall pressure vessel (Section 4.17 Norton)

r = nominal radius t = wall thickness L = length p = pressure

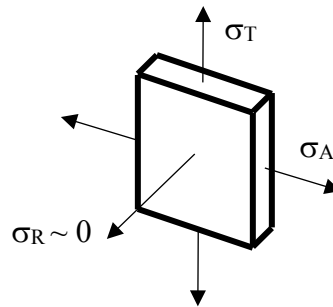
$t < r/10$

tangential (hoop) $\sigma_T = \frac{p(2rL)}{2Lt} = \frac{pr}{t}$ (sidewalls only)

axial $\sigma_A = \frac{p(\pi r^2)}{2\pi r t} = \frac{pr}{2t}$



also valid for hemispherical ends



combined stress

